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Index to drawing book 7 : floor panels

Include work order no. and floor no. if applicable

Roll # (e.g., CORR-4): DB06	Doc. ID (report #, dwg. #):	
Document Date:	Document Author Name:	Author Organization:

KEYWORDS – HIGHLIGHT ALL THAT APPLY

Level 1

Bankers Trust	Project 6	WTC 4 - South Plaza Bldg.
Concourse	Project 7	WTC 5 - North Plaza Bldg.
Electrical Substation	Project 8	WTC 6 - Customs House
Liberty Plaza	WFC 1 - Oppenhiemer Dow	WTC 7
PATH Station	WFC 2 - Tower B	WTC Complex
Project 1	WFC 3 - American Express	WTC Plaza
Project 2	WFC 4 - Tower D	
Project 3	WTC 1 - North Tower	Add
Project 4	WTC 2 - South Tower	
Project 5	WTC 3 - Marriot Hotel	

Level 2

Accident analysis & risk	Fire protection	Police	Add
Architectural	Fire service	Security	
Data request	Management	Structural	
Emergency management	Mech./elec.	Tenant alterations	
Evacuation	News coverage		

Level 3

1993 explosion	Deflection	Human comfort	Smoke control
A/E fitout	Demolition	HVAC	Smoke test
A-242	Design	Index	Spandrel beam
A-36	Detailing	Inspection	Specifications Splices
A-497	Doors	Insulation	Sprinkler
Air cooled condensers	Drawings	Job number	Stability
Analysis	Eastern States Steel	Joist	Stairways
Angles	Egress	L-50	Standards
Antenna	Elevator	Lighting	Steel
Artifacts	Emergency communications	Loads	Steel composition
Asbestos	Equipment	Maintenance	Stiffness
Beams	Erection marks	Marking	Structural review
Bow tie	Escalator	Mesh reinforcement	Survivor
Bracing	Expansion joint	Mill inspection	Tenant list
Bridging truss	Exterior columns	Model	Testing
Calculations	Fabrication	Mullion	Tie
Carpet	Family member	Operations	Tolerances
Ceilings	Field survey	Paint	Triad
Certification	Fire alarms	Partition layout	Truss
Chillers	Fireproofing	Photos	Vestibule
Cladding	Fire spread	Pipes	Vibration
Collapse	Flammability	Plumbing	Walls
Columns	Floor	Probability, uncertainty	Water supply
Communication systems	Floor load	Quality control	Weather
Composite truss	Floor slab	Reinforcing (strengthening)	Weight
Computer output	Floor system	Renovation	Weld detail
Computer program	Floor trusses	Repair	Weld size
Concrete	Foundations	Research & Development	Welding
Connections	Fracture	Roof	Wind
Construction	Frame	Safety	Windows
Contract	Frame analysis	Schedule	Yield strength
Core beams	Frequency	Sculpture	
Core columns	Fuel	Security system	Add
Cranes	Furniture layout	Shear knuckle	
Criteria	H Generators	Shear test	
Dampers	Guidelines	Shop drawings	
Debris	Gypsum board	Shortening	
Deck	Hat truss	Slurry wall	

WTCI-702-LERA

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS | DATE: 11/26/71

REVISIONS

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7-AB1-	31	1/ 6/69	7-AB3-	12	10/15/68	7-AB3-	62	1/20/69	7-AB4-	17.6	10/ 4/68	7-AB4-	49	7/22/68
7-AB1-	32	1/ 6/69	7-AB3-	13	10/15/68	7-AB3-	63	1/20/69	7-AB4-	17.7	10/ 4/68	7-AB4-	49.1	10/15/68
7-AB1-	33	1/ 6/69	7-AB3-	14	10/15/68	7-AB3-	70	11/25/68	7-AB4-	17.8	8/15/68	7-AB4-	49.2	10/ 4/68
7-AB1-	34	1/ 6/69	7-AB3-	15	7/22/68	7-AB3-	71	11/25/68	7-AB4-	18	1/ 6/69	7-AB4-	50	8/ 1/68
7-AB1-	35	1/ 6/69	7-AB3-	16	7/22/68	7-AB3-	72	11/25/68	7-AB4-	18.1	7/22/68	7-AB4-	51	8/ 1/68
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7-AB1-	44	11/25/68	7-AB3-	18	7/22/68	7-AB3-	74	11/25/68	7-AB4-	18.3	12/20/68	7-AB4-	53	10/ 4/68
7-AB1-	45	11/25/68	7-AB3-	19	7/22/68	7-AB3-	75	11/25/68	7-AB4-	18.4	10/ 4/68	7-AB4-	54	10/ 4/68
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7-AB1-	48	1/ 6/69	7-AB3-	22	7/22/68	7-AB3-	78	1/ 6/69	7-AB4-	21	1/ 6/69	7-AB4-	57	10/ 4/68
7-AB1-	52	2/15/68	7-AB3-	23	7/22/68	7-AB3-	79	10/15/68	7-AB4-	22	1/ 6/69	7-AB4-	58	10/ 4/68
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			7-AB3-	28	10/ 4/68	7-AB4-	2	1/ 6/69	7-AB4-	28	7/22/68			

LARRY E. ROBERTSON — LICENSED PROFESSIONAL ENGINEER — No. 39360 — STATE OF NEW YORK

702-1

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers — Printed

PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS | DATE: 10/ 1/69

REVISIONS

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7-AB1- 32	1/ 6/69	7-AB3- 13	10/15/68	7-AB3- 63	1/20/69	7-AB4- 17.7	10/ 4/68	7-AB4- 49.1	10/15/68
7-AB1- 33	1/ 6/69	7-AB3- 14	10/15/68	7-AB3- 70	11/25/68	7-AB4- 17.8	8/15/68	7-AB4- 49.2	10/ 4/68
7-AB1- 34	1/ 6/69	7-AB3- 15	7/22/68	7-AB3- 71	11/25/68	7-AB4- 18	1/ 6/69	7-AB4- 50	8/ 1/68
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		7-AB3- 27	10/ 4/68	7-AB4- 1	5/ 1/69	7-AB4- 25	7/22/68	7-AB4- 62	1/ 6/69
		7-AB3- 28	10/ 4/68	7-AB4- 2	1/ 6/69	7-AB4- 28	7/22/68		
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LICENSED PROFESSIONAL ENGINEER — LESLIE E. ROBERTSON — No. 34360 — UNIVERSITY OF THE STATE OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON Structural & Civil Engineers

Printed

PROJECT THE WORLD TRADE CENTER | TITLE FLOOR PANELS | DATE 9/ 2/69

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PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS | DATE: 7/ 1/69

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7-AB1- 26	1/ 6/69	7-AB3- 7	10/15/68	7-AB3- 57	1/ 6/69	7-AB4- 17.1	10/ 4/68	7-AB4- 46.1	7/22/68						
7-AB1- 27	1/ 6/69	7-AB3- 8	10/15/68	7-AB3- 58	1/ 6/69	7-AB4- 17.2	12/20/68	7-AB4- 47	7/22/68						
7-AB1- 28	1/ 6/69	7-AB3- 9	10/15/68	7-AB3- 59	1/ 6/69	7-AB4- 17.3	10/ 4/68	7-AB4- 47.1	10/15/68						
7-AB1- 29	1/ 6/69	7-AB3- 10	10/15/68	7-AB3- 60	1/ 6/69	7-AB4- 17.4	10/ 4/68	7-AB4- 48	1/ 3/68						
7-AB1- 30	1/ 6/69	7-AB3- 11	10/15/68	7-AB3- 61	1/ 6/69	7-AB4- 17.5	10/ 4/68	7-AB4- 49	7/22/68						
7-AB1- 31	1/ 6/69	7-AB3- 12	10/15/68	7-AB3- 62	1/20/69	7-AB4- 17.6	10/ 4/68	7-AB4- 49.1	10/15/68						
7-AB1- 32	1/ 6/69	7-AB3- 13	10/15/68	7-AB3- 63	1/20/69	7-AB4- 17.7	10/ 4/68	7-AB4- 49.2	10/ 4/68						
7-AB1- 33	1/ 6/69	7-AB3- 14	10/15/68	7-AB3- 70	11/25/68	7-AB4- 18	1/ 6/69	7-AB4- 50	8/ 1/68						
7-AB1- 34	1/ 6/69	7-AB3- 15	7/22/68	7-AB3- 71	11/25/68	7-AB4- 18.1	7/22/68	7-AB4- 51	8/ 1/68						
7-AB1- 35	1/ 6/69	7-AB3- 16	7/22/68	7-AB3- 72	11/25/68	7-AB4- 18.2	7/22/68	7-AB4- 52	8/ 1/68						
7-AB1- 36	1/ 6/69	7-AB3- 17	7/22/68	7-AB3- 73	11/25/68	7-AB4- 18.3	12/20/68	7-AB4- 53	10/ 4/68						
7-AB1- 44	11/25/68	7-AB3- 18	7/22/68	7-AB3- 74	11/25/68	7-AB4- 18.4	10/ 4/68	7-AB4- 54	10/ 4/68						
7-AB1- 45	11/25/68	7-AB3- 19	7/22/68	7-AB3- 75	11/25/68	7-AB4- 18.5	1/ 6/69	7-AB4- 55	10/ 4/68						
7-AB1- 46	11/25/68	7-AB3- 20	7/22/68	7-AB3- 76	11/25/68	7-AB4- 19	12/20/68	7-AB4- 56	10/ 4/68						
7-AB1- 47	11/25/68	7-AB3- 21	7/22/68	7-AB3- 77	1/ 6/69	7-AB4- 21	1/ 6/69	7-AB4- 57	10/ 4/68						
7-AB1- 48	1/ 6/69	7-AB3- 22	7/22/68	7-AB3- 78	1/ 6/69	7-AB4- 22	1/ 6/69	7-AB4- 58	10/ 4/68						
7-AB1- 52	2/15/68	7-AB3- 23	7/22/68	7-AB3- 79	10/15/68	7-AB4- 22.1	7/22/68	7-AB4- 59	10/ 4/68						
7-AB1- 53	7/22/68	7-AB3- 24	8/15/68	7-AB3- 80	10/15/68	7-AB4- 23	7/22/68	7-AB4- 60	10/ 4/68						
7-AB1- 54	1/ 6/69	7-AB3- 25	10/ 4/68			7-AB4- 24	10/15/68	7-AB4- 61	10/ 4/68						
7-AB1- 55	10/15/68	7-AB3- 26	10/ 4/68			7-AB4- 25	7/22/68	7-AB4- 62	1/ 6/69						
		7-AB3- 27	10/ 4/68	7-AB4- 1	4/ 1/69	7-AB4- 28	7/22/68								
		7-AB3- 28	10/ 4/68	7-AB4- 2	1/ 6/69	7-AB4- 28.1	7/22/68								
7-AB2- 1	4/ 1/69	7-AB3- 29	10/ 4/68	7-AB4- 2.1	1/ 6/69	7-AB4- 29	1/ 3/68								
7-AB2- 2	7/22/68	7-AB3- 30	10/ 4/68	7-AB4- 2.2	1/ 6/69	7-AB4- 30	4/ 1/68								
7-AB2- 3	7/22/68	7-AB3- 31	10/ 4/68	7-AB4- 2.3	1/ 6/69	7-AB4- 31	4/ 1/68								

LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
No. 3430
REGISTERED TO PRACTICE IN THE STATE OF NEW YORK

REVISIONS

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON Structural & Civil Engineers

PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS | DATE: 4/ 1/69

7-AB1- 1	4/ 1/69	7-AB2- 4	7/22/68	7-AB3- 32	10/ 4/68	7-AB4- 2.4	1/ 6/69	7-AB4- 31.1	4/ 1/68		
7-AB1- 2	7/22/68	7-AB2- 5	7/22/68	7-AB3- 33	10/ 4/68	7-AB4- 3	10/15/68	7-AB4- 31.2	1/20/69		
7-AB1- 3	7/22/68	7-AB2- 6	7/22/68	7-AB3- 34	10/ 4/68	7-AB4- 3.1	1/ 3/68	7-AB4- 32	4/ 1/68		
7-AB1- 4	7/22/68	7-AB2- 7	7/22/68	7-AB3- 35	10/ 4/68	7-AB4- 4	10/15/68	7-AB4- 32.1	4/ 1/68		
7-AB1- 5	10/15/68	7-AB2- 8	7/22/68	7-AB3- 36	10/ 4/68	7-AB4- 5	7/22/68	7-AB4- 33	4/ 1/68		
7-AB1- 6	7/22/68	7-AB2- 9	1/ 6/69	7-AB3- 37	10/ 4/68	7-AB4- 6	10/15/68	7-AB4- 34	10/15/68		
7-AB1- 7	7/22/68	7-AB2- 10	7/22/68	7-AB3- 38	10/ 4/68	7-AB4- 7	1/ 3/68	7-AB4- 34.1	1/20/69		
7-AB1- 8	7/22/68	7-AB2- 11	7/22/68	7-AB3- 39	10/ 4/68	7-AB4- 8	7/22/68	7-AB4- 35	10/15/68		
7-AB1- 9	7/22/68	7-AB2- 12	10/ 4/68	7-AB3- 40	10/ 4/68	7-AB4- 9	7/22/68	7-AB4- 36	10/15/68		
7-AB1- 10	7/22/68	7-AB2- 13	10/ 4/68	7-AB3- 41	1/ 6/69	7-AB4- 10	12/20/68	7-AB4- 37	10/15/68		
7-AB1- 11	7/22/68	7-AB2- 14	10/ 4/68	7-AB3- 42	1/ 6/69	7-AB4- 11	7/22/68	7-AB4- 38	10/15/68		
7-AB1- 12	7/22/68	7-AB2- 15	10/ 4/68	7-AB3- 43	1/ 6/69	7-AB4- 11.1	7/22/68	7-AB4- 38.1	10/15/68		
7-AB1- 13	10/ 4/68	7-AB2- 16	1/ 6/69	7-AB3- 44	1/ 6/69	7-AB4- 11.2	7/22/68	7-AB4- 38.2	10/15/68		
7-AB1- 14	10/ 4/68	7-AB2- 17	10/ 4/68	7-AB3- 45	1/ 6/69	7-AB4- 11.3	7/22/68	7-AB4- 38.3	1/20/69		
7-AB1- 15	10/ 4/68	7-AB2- 18	10/ 4/68	7-AB3- 46	1/ 6/69	7-AB4- 12	7/22/68	7-AB4- 39	7/22/68		
7-AB1- 16	10/ 4/68	7-AB2- 19	1/ 6/69	7-AB3- 47	1/ 6/69	7-AB4- 13	7/22/68	7-AB4- 40	7/22/68		
7-AB1- 17	10/ 4/68	7-AB2- 20	1/ 6/69	7-AB3- 48	1/ 6/69	7-AB4- 13.1	7/22/68	7-AB4- 41	7/22/68		
7-AB1- 18	10/ 4/68			7-AB3- 49	1/ 6/69	7-AB4- 13.2	10/15/68	7-AB4- 41.1	7/22/68		
7-AB1- 19	10/ 4/68			7-AB3- 50	1/ 6/69	7-AB4- 14	10/15/68	7-AB4- 42	10/15/68		
7-AB1- 20	10/ 4/68	7-AB3- 1	4/ 1/69	7-AB3- 51	1/ 6/69	7-AB4- 14.1	10/15/68	7-AB4- 42.1	10/15/68		
7-AB1- 21	10/ 4/68	7-AB3- 2	1/ 3/68	7-AB3- 52	1/ 6/69	7-AB4- 15	7/22/68	7-AB4- 43	7/22/68		
7-AB1- 22	10/ 4/68	7-AB3- 3	10/15/68	7-AB3- 53	1/ 6/69	7-AB4- 15.1	10/ 4/68	7-AB4- 44	1/ 3/68		
7-AB1- 23	10/ 4/68	7-AB3- 4	10/15/68	7-AB3- 54	1/ 6/69	7-AB4- 15.2	10/ 4/68	7-AB4- 45	12/20/68		
7-AB1- 24	1/ 6/69	7-AB3- 5	10/15/68	7-AB3- 55	1/ 6/69	7-AB4- 16	7/22/68	7-AB4- 45.1	11/25/68		
7-AB1- 25	1/ 6/69	7-AB3- 6	10/15/68	7-AB3- 56	1/ 6/69	7-AB4- 17	10/ 4/68	7-AB4- 46	10/15/68		
7-AB1- 26	1/ 6/69	7-AB3- 7	10/15/68	7-AB3- 57	1/ 6/69	7-AB4- 17.1	10/ 4/68	7-AB4- 46.1	7/22/68		
7-AB1- 27	1/ 6/69	7-AB3- 8	10/15/68	7-AB3- 58	1/ 6/69	7-AB4- 17.2	12/20/68	7-AB4- 47	7/22/68		
7-AB1- 28	1/ 6/69	7-AB3- 9	10/15/68	7-AB3- 59	1/ 6/69	7-AB4- 17.3	10/ 4/68	7-AB4- 47.1	10/15/68		
7-AB1- 29	1/ 6/69	7-AB3- 10	10/15/68	7-AB3- 60	1/ 6/69	7-AB4- 17.4	10/ 4/68	7-AB4- 48	1/ 3/68		
7-AB1- 30	1/ 6/69	7-AB3- 11	10/15/68	7-AB3- 61	1/ 6/69	7-AB4- 17.5	10/ 4/68	7-AB4- 49	7/22/68		
7-AB1- 31	1/ 6/69	7-AB3- 12	10/15/68	7-AB3- 62	1/20/69	7-AB4- 17.6	10/ 4/68	7-AB4- 49.1	10/15/68		
7-AB1- 32	1/ 6/69	7-AB3- 13	10/15/68	7-AB3- 63	1/20/69	7-AB4- 17.7	10/ 4/68	7-AB4- 49.2	10/ 4/68		
7-AB1- 33	1/ 6/69	7-AB3- 14	10/15/68	7-AB3- 70	11/25/68	7-AB4- 18	1/ 6/69	7-AB4- 50	8/ 1/68		
7-AB1- 34	1/ 6/69	7-AB3- 15	7/22/68	7-AB3- 71	11/25/68	7-AB4- 18.1	7/22/68	7-AB4- 51	8/ 1/68		
7-AB1- 35	1/ 6/69	7-AB3- 16	7/22/68	7-AB3- 72	11/25/68	7-AB4- 18.2	7/22/68	7-AB4- 52	8/ 1/68		
7-AB1- 36	1/ 6/69	7-AB3- 17	7/22/68	7-AB3- 73	11/25/68	7-AB4- 18.3	12/20/68	7-AB4- 53	10/ 4/68		
7-AB1- 44	11/25/68	7-AB3- 18	7/22/68	7-AB3- 74	11/25/68	7-AB4- 18.4	10/ 4/68	7-AB4- 54	10/ 4/68		
7-AB1- 45	11/25/68	7-AB3- 19	7/22/68	7-AB3- 75	11/25/68	7-AB4- 18.5	1/ 6/69	7-AB4- 55	10/ 4/68		
7-AB1- 46	11/25/68	7-AB3- 20	7/22/68	7-AB3- 76	11/25/68	7-AB4- 19	12/20/68	7-AB4- 56	10/ 4/68		
7-AB1- 47	11/25/68	7-AB3- 21	7/22/68	7-AB3- 77	1/ 6/69	7-AB4- 21	1/ 6/69	7-AB4- 57	10/ 4/68		
7-AB1- 48	1/ 6/69	7-AB3- 22	7/22/68	7-AB3- 78	1/ 6/69	7-AB4- 22	1/ 6/69	7-AB4- 58	10/ 4/68		
7-AB1- 52	2/15/68	7-AB3- 23	7/22/68	7-AB3- 79	10/15/68	7-AB4- 22.1	7/22/68	7-AB4- 59	10/ 4/68		
7-AB1- 53	7/22/68	7-AB3- 24	8/15/68	7-AB3- 80	10/15/68	7-AB4- 23	7/22/68	7-AB4- 60	10/ 4/68		
7-AB1- 54	1/ 6/69	7-AB3- 25	10/ 4/68			7-AB4- 24	10/15/68	7-AB4- 61	10/ 4/68		
7-AB1- 55	10/15/68	7-AB3- 26	10/ 4/68			7-AB4- 25	7/22/68	7-AB4- 62	1/ 6/69		
		7-AB3- 27	10/ 4/68	7-AB4- 1	4/ 1/69	7-AB4- 28	7/22/68				
		7-AB3- 28	10/ 4/68	7-AB4- 2	1/ 6/69	7-AB4- 28.1	7/22/68				
7-AB2- 1	4/ 1/69	7-AB3- 29	10/ 4/68	7-AB4- 2.1	1/ 6/69	7-AB4- 29	1/ 3/68				
7-AB2- 2	7/22/68	7-AB3- 30	10/ 4/68	7-AB4- 2.2	1/ 6/69	7-AB4- 30	4/ 1/68				
7-AB2- 3	7/22/68	7-AB3- 31	10/ 4/68	7-AB4- 2.3	1/ 6/69	7-AB4- 31	4/ 1/68				

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PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS | DATE: 3/ 3/69

REVISIONS

7-AB1-	1	3/ 3/69	7-AB2-	4	7/22/68	7-AB3-	32	10/ 4/68	7-AB4-	2.6	1/ 6/69	7-AB4-	31.1	4/ 1/68	
7-AB1-	2	7/22/68	7-AB2-	5	7/22/68	7-AB3-	33	10/ 4/68	7-AB4-	3	10/15/68	7-AB4-	31.2	1/20/69	
7-AB1-	3	7/22/68	7-AB2-	6	7/22/68	7-AB3-	34	10/ 4/68	7-AB4-	3.1	1/ 3/68	7-AB4-	32	4/ 1/68	
7-AB1-	4	7/22/68	7-AB2-	7	7/22/68	7-AB3-	35	10/ 4/68	7-AB4-	4	10/15/68	7-AB4-	32.1	4/ 1/68	
7-AB1-	5	10/15/68	7-AB2-	8	7/22/68	7-AB3-	36	10/ 4/68	7-AB4-	5	7/22/68	7-AB4-	33	4/ 1/68	
7-AB1-	6	7/22/68	7-AB2-	9	1/ 6/69	7-AB3-	37	10/ 4/68	7-AB4-	6	10/15/68	7-AB4-	34	10/15/68	
7-AB1-	7	7/22/68	7-AB2-	10	7/22/68	7-AB3-	38	10/ 4/68	7-AB4-	7	1/ 3/68	7-AB4-	34.1	1/20/69	
7-AB1-	8	7/22/68	7-AB2-	11	7/22/68	7-AB3-	39	10/ 4/68	7-AB4-	8	7/22/68	7-AB4-	35	10/15/68	
7-AB1-	9	7/22/68	7-AB2-	12	10/ 4/68	7-AB3-	40	10/ 4/68	7-AB4-	9	7/22/68	7-AB4-	36	10/15/68	
7-AB1-	10	7/22/68	7-AB2-	13	10/ 4/68	7-AB3-	41	1/ 6/69	7-AB4-	10	12/20/68	7-AB4-	37	10/15/68	
7-AB1-	11	7/22/68	7-AB2-	14	10/ 4/68	7-AB3-	42	1/ 6/69	7-AB4-	11	7/22/68	7-AB4-	38	10/15/68	
7-AB1-	12	7/22/68	7-AB2-	15	10/ 4/68	7-AB3-	43	1/ 6/69	7-AB4-	11.1	7/22/68	7-AB4-	38.1	10/15/68	
7-AB1-	13	10/ 4/68	7-AB2-	16	1/ 6/69	7-AB3-	44	1/ 6/69	7-AB4-	11.2	7/22/68	7-AB4-	38.2	10/15/68	
7-AB1-	14	10/ 4/68	7-AB2-	17	10/ 4/68	7-AB3-	45	1/ 6/69	7-AB4-	11.3	7/22/68	7-AB4-	38.3	1/20/69	
7-AB1-	15	10/ 4/68	7-AB2-	18	10/ 4/68	7-AB3-	46	1/ 6/69	7-AB4-	12	7/22/68	7-AB4-	39	7/22/68	
7-AB1-	16	10/ 4/68	7-AB2-	19	1/ 6/69	7-AB3-	47	1/ 6/69	7-AB4-	13	7/22/68	7-AB4-	40	7/22/68	
7-AB1-	17	10/ 4/68	7-AB2-	20	1/ 6/69	7-AB3-	48	1/ 6/69	7-AB4-	13.1	7/22/68	7-AB4-	41	7/22/68	
7-AB1-	18	10/ 4/68				7-AB3-	49	1/ 6/69	7-AB4-	13.2	10/15/68	7-AB4-	41.1	7/22/68	
7-AB1-	19	10/ 4/68				7-AB3-	50	1/ 6/69	7-AB4-	14	10/15/68	7-AB4-	42	10/15/68	
7-AB1-	20	10/ 4/68	7-AB3-	1	3/ 3/69	7-AB3-	51	1/ 6/69	7-AB4-	14.1	10/15/68	7-AB4-	42.1	10/15/68	
7-AB1-	21	10/ 4/68	7-AB3-	2	1/ 3/68	7-AB3-	52	1/ 6/69	7-AB4-	15	7/22/68	7-AB4-	43	7/22/68	
7-AB1-	22	10/ 4/68	7-AB3-	3	10/15/68	7-AB3-	53	1/ 6/69	7-AB4-	15.1	10/ 4/68	7-AB4-	44	1/ 3/68	
7-AB1-	23	10/ 4/68	7-AB3-	4	10/15/68	7-AB3-	54	1/ 6/69	7-AB4-	15.2	10/ 4/68	7-AB4-	45	12/20/68	
7-AB1-	24	1/ 6/69	7-AB3-	5	10/15/68	7-AB3-	55	1/ 6/69	7-AB4-	16	7/22/68	7-AB4-	45.1	11/25/68	
7-AB1-	25	1/ 6/69	7-AB3-	6	10/15/68	7-AB3-	56	1/ 6/69	7-AB4-	17	10/ 4/68	7-AB4-	46	10/15/68	
7-AB1-	26	1/ 6/69	7-AB3-	7	10/15/68	7-AB3-	57	1/ 6/69	7-AB4-	17.1	10/ 4/68	7-AB4-	46.1	7/22/68	
7-AB1-	27	1/ 6/69	7-AB3-	8	10/15/68	7-AB3-	58	1/ 6/69	7-AB4-	17.2	12/20/68	7-AB4-	47	7/22/68	
7-AB1-	28	1/ 6/69	7-AB3-	9	10/15/68	7-AB3-	59	1/ 6/69	7-AB4-	17.3	10/ 4/68	7-AB4-	47.1	10/15/68	
7-AB1-	29	1/ 6/69	7-AB3-	10	10/15/68	7-AB3-	60	1/ 6/69	7-AB4-	17.4	10/ 4/68	7-AB4-	48	1/ 3/68	
7-AB1-	30	1/ 6/69	7-AB3-	11	10/15/68	7-AB3-	61	1/ 6/69	7-AB4-	17.5	10/ 4/68	7-AB4-	49	7/22/68	
7-AB1-	31	1/ 6/69	7-AB3-	12	10/15/68	7-AB3-	62	1/20/69	7-AB4-	17.6	10/ 4/68	7-AB4-	49.1	10/15/68	
7-AB1-	32	1/ 6/69	7-AB3-	13	10/15/68	7-AB3-	63	1/20/69	7-AB4-	17.7	10/ 4/68	7-AB4-	49.2	10/ 4/68	
7-AB1-	33	1/ 6/69	7-AB3-	14	10/15/68	7-AB3-	70	11/25/68	7-AB4-	18	1/ 6/69	7-AB4-	50	8/ 1/68	
7-AB1-	34	1/ 6/69	7-AB3-	15	7/22/68	7-AB3-	71	11/25/68	7-AB4-	18.1	7/22/68	7-AB4-	51	8/ 1/68	
7-AB1-	35	1/ 6/69	7-AB3-	16	7/22/68	7-AB3-	72	11/25/68	7-AB4-	18.2	7/22/68	7-AB4-	52	8/ 1/68	
7-AB1-	36	1/ 6/69	7-AB3-	17	7/22/68	7-AB3-	73	11/25/68	7-AB4-	18.3	12/20/68	7-AB4-	53	10/ 4/68	
7-AB1-	44	11/25/68	7-AB3-	18	7/22/68	7-AB3-	74	11/25/68	7-AB4-	18.4	10/ 4/68	7-AB4-	54	10/ 4/68	
7-AB1-	45	11/25/68	7-AB3-	19	7/22/68	7-AB3-	75	11/25/68	7-AB4-	18.5	1/ 6/69	7-AB4-	55	10/ 4/68	
7-AB1-	46	11/25/68	7-AB3-	20	7/22/68	7-AB3-	76	11/25/68	7-AB4-	19	12/20/68	7-AB4-	56	10/ 4/68	
7-AB1-	47	11/25/68	7-AB3-	21	7/22/68	7-AB3-	77	1/ 6/69	7-AB4-	21	1/ 6/69	7-AB4-	57	10/ 4/68	
7-AB1-	48	1/ 6/69	7-AB3-	22	7/22/68	7-AB3-	78	1/ 6/69	7-AB4-	22	1/ 6/69	7-AB4-	58	10/ 4/68	
7-AB1-	52	2/15/68	7-AB3-	23	7/22/68	7-AB3-	79	10/15/68	7-AB4-	22.1	7/22/68	7-AB4-	59	10/ 4/68	
7-AB1-	53	7/22/68	7-AB3-	24	8/15/68	7-AB3-	80	10/15/68	7-AB4-	23	7/22/68	7-AB4-	60	10/ 4/68	
7-AB1-	54	1/ 6/69	7-AB3-	25	10/ 4/68				7-AB4-	24	10/15/68	7-AB4-	61	10/ 4/68	
7-AB1-	55	10/15/68	7-AB3-	26	10/ 4/68				7-AB4-	25	7/22/68	7-AB4-	62	1/ 6/69	
			7-AB3-	27	10/ 4/68	7-AB4-	1	3/ 3/69	7-AB4-	28	7/22/68				
			7-AB3-	28	10/ 4/68	7-AB4-	2	1/ 6/69	7-AB4-	28.1	7/22/68				
7-AB2-	1	3/ 3/69	7-AB3-	29	10/ 4/68	7-AB4-	2.1	1/ 6/69	7-AB4-	29	1/ 3/68				
7-AB2-	2	7/22/68	7-AB3-	30	10/ 4/68	7-AB4-	2.2	1/ 6/69	7-AB4-	30	4/ 1/68				
7-AB2-	3	7/22/68	7-AB3-	31	10/ 4/68	7-AB4-	2.3	1/ 6/69	7-AB4-	31	4/ 1/68				



SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS | DATE: 2/ 3/69

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7-AB1- 1	2/ 3/69	7-AB2- 4	7/22/68	7-AB3- 32	10/ 4/68	7-AB4- 2.4	1/ 6/69	7-AB4- 31.1	4/ 1/68
7-AB1- 2	7/22/68	7-AB2- 5	7/22/68	7-AB3- 33	10/ 4/68	7-AB4- 3	10/15/68	7-AB4- 31.2	1/20/69
7-AB1- 3	7/22/68	7-AB2- 6	7/22/68	7-AB3- 34	10/ 4/68	7-AB4- 3.1	1/ 3/68	7-AB4- 32	4/ 1/68
7-AB1- 4	7/22/68	7-AB2- 7	7/22/68	7-AB3- 35	10/ 4/68	7-AB4- 4	10/15/68	7-AB4- 32.1	4/ 1/68
7-AB1- 5	10/15/68	7-AB2- 8	7/22/68	7-AB3- 36	10/ 4/68	7-AB4- 5	7/22/68	7-AB4- 33	4/ 1/68
7-AB1- 6	7/22/68	7-AB2- 9	1/ 6/69	7-AB3- 37	10/ 4/68	7-AB4- 6	10/15/68	7-AB4- 34	10/15/68
7-AB1- 7	7/22/68	7-AB2- 10	7/22/68	7-AB3- 38	10/ 4/68	7-AB4- 7	1/ 3/68	7-AB4- 34.1	1/20/69
7-AB1- 8	7/22/68	7-AB2- 11	7/22/68	7-AB3- 39	10/ 4/68	7-AB4- 8	7/22/68	7-AB4- 35	10/15/68
7-AB1- 9	7/22/68	7-AB2- 12	10/ 4/68	7-AB3- 40	10/ 4/68	7-AB4- 9	7/22/68	7-AB4- 36	10/15/68
7-AB1- 10	7/22/68	7-AB2- 13	10/ 4/68	7-AB3- 41	1/ 6/69	7-AB4- 10	12/20/68	7-AB4- 37	10/15/68
7-AB1- 11	7/22/68	7-AB2- 14	10/ 4/68	7-AB3- 42	1/ 6/69	7-AB4- 11	7/22/68	7-AB4- 38	10/15/68
7-AB1- 12	7/22/68	7-AB2- 15	10/ 4/68	7-AB3- 43	1/ 6/69	7-AB4- 11.1	7/22/68	7-AB4- 38.1	10/15/68
7-AB1- 13	10/ 4/68	7-AB2- 16	1/ 6/69	7-AB3- 44	1/ 6/69	7-AB4- 11.2	7/22/68	7-AB4- 38.2	10/15/68
7-AB1- 14	10/ 4/68	7-AB2- 17	10/ 4/68	7-AB3- 45	1/ 6/69	7-AB4- 11.3	7/22/68	7-AB4- 38.3	1/20/69
7-AB1- 15	10/ 4/68	7-AB2- 18	10/ 4/68	7-AB3- 46	1/ 6/69	7-AB4- 12	7/22/68	7-AB4- 39	7/22/68
7-AB1- 16	10/ 4/68	7-AB2- 19	1/ 6/69	7-AB3- 47	1/ 6/69	7-AB4- 13	7/22/68	7-AB4- 40	7/22/68
7-AB1- 17	10/ 4/68	7-AB2- 20	1/ 6/69	7-AB3- 48	1/ 6/69	7-AB4- 13.1	7/22/68	7-AB4- 41	7/22/68
7-AB1- 18	10/ 4/68			7-AB3- 49	1/ 6/69	7-AB4- 13.2	10/15/68	7-AB4- 41.1	7/22/68
7-AB1- 19	10/ 4/68			7-AB3- 50	1/ 6/69	7-AB4- 14	10/15/68	7-AB4- 42	10/15/68
7-AB1- 20	10/ 4/68	7-AB3- 1	2/ 3/69	7-AB3- 51	1/ 6/69	7-AB4- 14.1	10/15/68	7-AB4- 42.1	10/15/68
7-AB1- 21	10/ 4/68	7-AB3- 2	1/ 3/68	7-AB3- 52	1/ 6/69	7-AB4- 15	7/22/68	7-AB4- 43	7/22/68
7-AB1- 22	10/ 4/68	7-AB3- 3	10/15/68	7-AB3- 53	1/ 6/69	7-AB4- 15.1	10/ 4/68	7-AB4- 44	1/ 3/68
7-AB1- 23	10/ 4/68	7-AB3- 4	10/15/68	7-AB3- 54	1/ 6/69	7-AB4- 15.2	10/ 4/68	7-AB4- 45	12/20/68
7-AB1- 24	1/ 6/69	7-AB3- 5	10/15/68	7-AB3- 55	1/ 6/69	7-AB4- 16	7/22/68	7-AB4- 45.1	11/25/68
7-AB1- 25	1/ 6/69	7-AB3- 6	10/15/68	7-AB3- 56	1/ 6/69	7-AB4- 17	10/ 4/68	7-AB4- 46	10/15/68
7-AB1- 26	1/ 6/69	7-AB3- 7	10/15/68	7-AB3- 57	1/ 6/69	7-AB4- 17.1	10/ 4/68	7-AB4- 46.1	7/22/68
7-AB1- 27	1/ 6/69	7-AB3- 8	10/15/68	7-AB3- 58	1/ 6/69	7-AB4- 17.2	12/20/68	7-AB4- 47	7/22/68
7-AB1- 28	1/ 6/69	7-AB3- 9	10/15/68	7-AB3- 59	1/ 6/69	7-AB4- 17.3	10/ 4/68	7-AB4- 47.1	10/15/68
7-AB1- 29	1/ 6/69	7-AB3- 10	10/15/68	7-AB3- 60	1/ 6/69	7-AB4- 17.4	10/ 4/68	7-AB4- 48	1/ 3/68
7-AB1- 30	1/ 6/69	7-AB3- 11	10/15/68	7-AB3- 61	1/ 6/69	7-AB4- 17.5	10/ 4/68	7-AB4- 49	7/22/68
7-AB1- 31	1/ 6/69	7-AB3- 12	10/15/68	7-AB3- 62	1/20/69	7-AB4- 17.6	10/ 4/68	7-AB4- 49.1	10/15/68
7-AB1- 32	1/ 6/69	7-AB3- 13	10/15/68	7-AB3- 63	1/20/69	7-AB4- 17.7	10/ 4/68	7-AB4- 49.2	10/ 4/68
7-AB1- 33	1/ 6/69	7-AB3- 14	10/15/68	7-AB3- 70	11/25/68	7-AB4- 18	1/ 6/69	7-AB4- 50	8/ 1/68
7-AB1- 34	1/ 6/69	7-AB3- 15	7/22/68	7-AB3- 71	11/25/68	7-AB4- 18.1	7/22/68	7-AB4- 51	8/ 1/68
7-AB1- 35	1/ 6/69	7-AB3- 16	7/22/68	7-AB3- 72	11/25/68	7-AB4- 18.2	7/22/68	7-AB4- 52	8/ 1/68
7-AB1- 36	1/ 6/69	7-AB3- 17	7/22/68	7-AB3- 73	11/25/68	7-AB4- 18.3	12/20/68	7-AB4- 53	10/ 4/68
7-AB1- 44	11/25/68	7-AB3- 18	7/22/68	7-AB3- 74	11/25/68	7-AB4- 18.4	10/ 4/68	7-AB4- 54	10/ 4/68
7-AB1- 45	11/25/68	7-AB3- 19	7/22/68	7-AB3- 75	11/25/68	7-AB4- 18.5	1/ 6/69	7-AB4- 55	10/ 4/68
7-AB1- 46	11/25/68	7-AB3- 20	7/22/68	7-AB3- 76	11/25/68	7-AB4- 19	12/20/68	7-AB4- 56	10/ 4/68
7-AB1- 47	11/25/68	7-AB3- 21	7/22/68	7-AB3- 77	1/ 6/69	7-AB4- 21	1/ 6/69	7-AB4- 57	10/ 4/68
7-AB1- 48	1/ 6/69	7-AB3- 22	7/22/68	7-AB3- 78	1/ 6/69	7-AB4- 22	1/ 6/69	7-AB4- 58	10/ 4/68
7-AB1- 52	2/15/68	7-AB3- 23	7/22/68	7-AB3- 79	10/15/68	7-AB4- 22.1	7/22/68	7-AB4- 59	10/ 4/68
7-AB1- 53	7/22/68	7-AB3- 24	8/15/68	7-AB3- 80	10/15/68	7-AB4- 23	7/22/68	7-AB4- 60	10/ 4/68
7-AB1- 54	1/ 6/69	7-AB3- 25	10/ 4/68			7-AB4- 24	10/15/68	7-AB4- 61	10/ 4/68
7-AB1- 55	10/15/68	7-AB3- 26	10/ 4/68			7-AB4- 25	7/22/68	7-AB4- 62	1/ 6/69
		7-AB3- 27	10/ 4/68	7-AB4- 1	2/ 3/69	7-AB4- 28	7/22/68		
		7-AB3- 28	10/ 4/68	7-AB4- 2	1/ 6/69	7-AB4- 28.1	7/22/68		
7-AB2- 1	2/ 3/69	7-AB3- 29	10/ 4/68	7-AB4- 2.1	1/ 6/69	7-AB4- 29	1/ 3/68		
7-AB2- 2	7/22/68	7-AB3- 30	10/ 4/68	7-AB4- 2.2	1/ 6/69	7-AB4- 30	4/ 1/68		
7-AB2- 3	7/22/68	7-AB3- 31	10/ 4/68	7-AB4- 2.3	1/ 6/69	7-AB4- 31	4/ 1/68		

LICENSED PROFESSIONAL ENGINEER — LESLIE E. ROBERTSON — No. 34560 — UNIVERSITY OF THE STATE OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS | DATE: 1/ 3/69

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7-AB1- 1	1/ 3/69	7-AB2- 18	10/ 4/68	7-AB3- 79	10/15/68	7-AB4- 23	7/22/68								
7-AB1- 2	7/22/68			7-AB3- 80	10/15/68	7-AB4- 24	10/15/68	7-AB5- 1	1/ 3/69						
7-AB1- 3	7/22/68					7-AB4- 25	7/22/68	7-AB5- 2	11/24/67						
7-AB1- 4	7/22/68	7-AB3- 1	1/ 3/69			7-AB4- 28	7/22/68	7-AB5- 3	6/17/68						
7-AB1- 5	10/15/68	7-AB3- 2	1/ 3/68	7-AB4- 1	1/ 3/69	7-AB4- 28.1	7/22/68	7-AB5- 3.1	6/17/68						
7-AB1- 6	7/22/68	7-AB3- 3	10/15/68	7-AB4- 2	7/22/68	7-AB4- 29	1/ 3/68	7-AB5- 3.2	6/17/68						
7-AB1- 7	7/22/68	7-AB3- 4	10/15/68	7-AB4- 2.1	7/22/68	7-AB4- 30	4/ 1/68	7-AB5- 4	10/11/68						
7-AB1- 8	7/22/68	7-AB3- 5	10/15/68	7-AB4- 2.2	10/ 4/68	7-AB4- 31	4/ 1/68	7-AB5- 4.1	10/18/67						
7-AB1- 9	7/22/68	7-AB3- 6	10/15/68	7-AB4- 2.3	10/ 4/68	7-AB4- 31.1	4/ 1/68	7-AB5- 5	6/17/68						
7-AB1- 10	7/22/68	7-AB3- 7	10/15/68	7-AB4- 2.4	8/15/68	7-AB4- 32	4/ 1/68	7-AB5- 6	6/17/68						
7-AB1- 11	7/22/68	7-AB3- 8	10/15/68	7-AB4- 3	10/15/68	7-AB4- 32.1	4/ 1/68	7-AB5- 7	6/17/68						
7-AB1- 12	7/22/68	7-AB3- 9	10/15/68	7-AB4- 3.1	1/ 3/68	7-AB4- 33	4/ 1/68	7-AB5- 8	6/17/68						
7-AB1- 13	10/ 4/68	7-AB3- 10	10/15/68	7-AB4- 4	10/15/68	7-AB4- 34	10/15/68	7-AB5- 8.1	6/17/68						
7-AB1- 14	10/ 4/68	7-AB3- 11	10/15/68	7-AB4- 5	7/22/68	7-AB4- 35	10/15/68	7-AB5- 8.2	6/17/68						
7-AB1- 15	10/ 4/68	7-AB3- 12	10/15/68	7-AB4- 6	10/15/68	7-AB4- 36	10/15/68	7-AB5- 9	6/17/68						
7-AB1- 16	10/ 4/68	7-AB3- 13	10/15/68	7-AB4- 7	1/ 3/68	7-AB4- 37	10/15/68	7-AB5- 10	6/17/68						
7-AB1- 17	10/ 4/68	7-AB3- 14	10/15/68	7-AB4- 8	7/22/68	7-AB4- 38	10/15/68	7-AB5- 11	6/17/68						
7-AB1- 18	10/ 4/68	7-AB3- 15	7/22/68	7-AB4- 9	7/22/68	7-AB4- 38.1	10/15/68	7-AB5- 12	6/17/68						
7-AB1- 19	10/ 4/68	7-AB3- 16	7/22/68	7-AB4- 10	12/20/68	7-AB4- 38.2	10/15/68	7-AB5- 13	6/17/68						
7-AB1- 20	10/ 4/68	7-AB3- 17	7/22/68	7-AB4- 11	7/22/68	7-AB4- 39	7/22/68	7-AB5- 14	6/17/68						
7-AB1- 21	10/ 4/68	7-AB3- 18	7/22/68	7-AB4- 11.1	7/22/68	7-AB4- 40	7/22/68	7-AB5- 15	6/17/68						
7-AB1- 22	10/ 4/68	7-AB3- 19	7/22/68	7-AB4- 11.2	7/22/68	7-AB4- 41	7/22/68	7-AB5- 16	11/24/67						
7-AB1- 23	10/ 4/68	7-AB3- 20	7/22/68	7-AB4- 11.3	7/22/68	7-AB4- 41.1	7/22/68	7-AB5- 17	6/17/68						
7-AB1- 44	11/25/68	7-AB3- 21	7/22/68	7-AB4- 12	7/22/68	7-AB4- 42	10/15/68	7-AB5- 18	6/17/68						
7-AB1- 45	11/25/68	7-AB3- 22	7/22/68	7-AB4- 13	7/22/68	7-AB4- 42.1	10/15/68	7-AB5- 19	6/17/68						
7-AB1- 46	11/25/68	7-AB3- 23	7/22/68	7-AB4- 13.1	7/22/68	7-AB4- 43	7/22/68								
7-AB1- 47	11/25/68	7-AB3- 24	8/15/68	7-AB4- 13.2	10/15/68	7-AB4- 44	1/ 3/68								
7-AB1- 48	11/25/68	7-AB3- 25	10/ 4/68	7-AB4- 14	10/15/68	7-AB4- 45	12/20/68								
7-AB1- 52	2/15/68	7-AB3- 26	10/ 4/68	7-AB4- 14.1	10/15/68	7-AB4- 45.1	11/25/68								
7-AB1- 53	7/22/68	7-AB3- 27	10/ 4/68	7-AB4- 15	7/22/68	7-AB4- 46	10/15/68								
7-AB1- 54	11/25/68	7-AB3- 28	10/ 4/68	7-AB4- 15.1	10/ 4/68	7-AB4- 46.1	7/22/68								
7-AB1- 55	10/15/68	7-AB3- 29	10/ 4/68	7-AB4- 15.2	10/ 4/68	7-AB4- 47	7/22/68								
		7-AB3- 30	10/ 4/68	7-AB4- 16	7/22/68	7-AB4- 47.1	10/15/68								
		7-AB3- 31	10/ 4/68	7-AB4- 17	10/ 4/68	7-AB4- 48	1/ 3/68								
7-AB2- 1	1/ 3/69	7-AB3- 32	10/ 4/68	7-AB4- 17.1	10/ 4/68	7-AB4- 49	7/22/68								
7-AB2- 2	7/22/68	7-AB3- 33	10/ 4/68	7-AB4- 17.2	12/20/68	7-AB4- 49.1	10/15/68								
7-AB2- 3	7/22/68	7-AB3- 34	10/ 4/68	7-AB4- 17.3	10/ 4/68	7-AB4- 49.2	10/ 4/68								
7-AB2- 4	7/22/68	7-AB3- 35	10/ 4/68	7-AB4- 17.4	10/ 4/68	7-AB4- 50	8/ 1/68								
7-AB2- 5	7/22/68	7-AB3- 36	10/ 4/68	7-AB4- 17.5	10/ 4/68	7-AB4- 51	8/ 1/68								
7-AB2- 6	7/22/68	7-AB3- 37	10/ 4/68	7-AB4- 17.6	10/ 4/68	7-AB4- 52	8/ 1/68								
7-AB2- 7	7/22/68	7-AB3- 38	10/ 4/68	7-AB4- 17.7	10/ 4/68	7-AB4- 53	10/ 4/68								
7-AB2- 8	7/22/68	7-AB3- 39	10/ 4/68	7-AB4- 18	7/22/68	7-AB4- 54	10/ 4/68								
7-AB2- 9	7/22/68	7-AB3- 40	10/ 4/68	7-AB4- 18.1	7/22/68	7-AB4- 55	10/ 4/68								
7-AB2- 10	7/22/68	7-AB3- 70	11/25/68	7-AB4- 18.2	7/22/68	7-AB4- 56	10/ 4/68								
7-AB2- 11	7/22/68	7-AB3- 71	11/25/68	7-AB4- 18.3	12/20/68	7-AB4- 57	10/ 4/68								
7-AB2- 12	10/ 4/68	7-AB3- 72	11/25/68	7-AB4- 18.4	10/ 4/68	7-AB4- 58	10/ 4/68								
7-AB2- 13	10/ 4/68	7-AB3- 73	11/25/68	7-AB4- 19	12/20/68	7-AB4- 59	10/ 4/68								
7-AB2- 14	10/ 4/68	7-AB3- 74	11/25/68	7-AB4- 21	7/22/68	7-AB4- 60	10/ 4/68								
7-AB2- 15	10/ 4/68	7-AB3- 75	11/25/68	7-AB4- 22	7/22/68	7-AB4- 61	10/ 4/68								

LICENSED PROFESSIONAL ENGINEER
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SKILLING-HELLE-CHRISTIANSEN-ROBERTSON — Civil & Structural Engineers

PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS | DATE: 5/10/68

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7-AB1-	1	4/ 1/68	7-AB3-	20	5/10/68	7-AB4-	32	4/ 1/68
7-AB1-	2	2/15/68	7-AB3-	21	5/10/68	7-AB4-	32.1	4/ 1/68
7-AB1-	3	4/15/68	7-AB3-	22	5/10/68	7-AB4-	33	4/ 1/68
7-AB1-	4	4/15/68	7-AB3-	23	5/10/68	7-AB4-	34	4/ 1/68
7-AB1-	5	4/15/68	7-AB3-	70	1/ 3/68	7-AB4-	35	4/ 1/68
7-AB1-	6	5/10/68	7-AB3-	79	4/ 1/68	7-AB4-	36	4/ 1/68
7-AB1-	7	5/10/68	7-AB3-	80	4/ 1/68	7-AB4-	37	4/ 1/68
7-AB1-	8	5/10/68				7-AB4-	38	4/ 1/68
7-AB1-	9	5/10/68				7-AB4-	38.1	4/ 1/68
7-AB1-	10	5/10/68	7-AB4-	1	4/ 1/68	7-AB4-	38.2	4/ 1/68
7-AB1-	11	4/ 1/68	7-AB4-	2	1/ 3/68	7-AB4-	39	1/ 3/68
7-AB1-	12	5/10/68	7-AB4-	2.1	1/ 3/68	7-AB4-	40	1/ 3/68
7-AB1-	52	2/15/68	7-AB4-	2.2	1/ 3/68	7-AB4-	41	1/ 3/68
7-AB1-	53	4/ 1/68	7-AB4-	2.3	1/ 3/68	7-AB4-	42	4/ 1/68
7-AB1-	54	4/15/68	7-AB4-	2.4	1/ 3/68	7-AB4-	42.1	4/ 1/68
7-AB1-	55	4/ 1/68	7-AB4-	3	1/ 3/68	7-AB4-	43	1/ 3/68
			7-AB4-	3.1	1/ 3/68	7-AB4-	44	1/ 3/68
			7-AB4-	4	1/ 3/68	7-AB4-	45	4/ 1/68
7-AB2-	1	4/ 1/68	7-AB4-	5	1/ 3/68	7-AB4-	46	4/ 1/68
7-AB2-	2	4/ 1/68	7-AB4-	6	1/ 3/68	7-AB4-	47	4/ 1/68
7-AB2-	3	4/15/68	7-AB4-	7	1/ 3/68	7-AB4-	47.1	4/ 1/68
7-AB2-	4	4/ 1/68 R	7-AB4-	10	1/ 3/68	7-AB4-	48	1/ 3/68
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7-AB2-	8	2/15/68	7-AB4-	11.3	5/ 6/68	7-AB5-	1	11/24/67
7-AB2-	9	4/ 1/68	7-AB4-	12	5/ 6/68	7-AB5-	2	11/24/67
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			7-AB4-	13.2	5/ 6/68	7-AB5-	4.1	10/18/67
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7-AB3-	1	4/ 1/68	7-AB4-	14.1	1/ 3/68	7-AB5-	6	11/24/67
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7-AB3-	3	4/ 1/68	7-AB4-	15.1	1/ 3/68	7-AB5-	8	11/24/67
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7-AB3-	10	4/15/68	7-AB4-	21	1/ 3/68	7-AB5-	14	11/24/67
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7-AB3-	18	5/10/68	7-AB4-	31	4/ 1/68			
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NOT FOR CONSTRUCTION

LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
No. 34360
DIVISION OF THE STATE OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT	TITLE	DATE
THE WORLD TRADE CENTER	FLOOR PANELS	11/26/71

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2-C/2 6/13/69
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7-C/2 6/13/69
8-C/1 5/14/69
8-C/2 6/13/69
9-C/1 5/20/69
9-C/2 6/13/69
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SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers — Printed

PROJECT	TITLE	DATE
THE WORLD TRADE CENTER	FLOOR PANELS	10/ 1/69

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7-AB5- 10	6/17/68
7-AB5- 11	6/17/68
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2-C/2	6/13/69
7-C/1	5/13/69
7-C/2	6/13/69
8-C/1	5/14/69
8-C/2	6/13/69
9-C/1	5/20/69
9-C/2	6/13/69
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LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
No. 34360
UNIVERSITY OF THE STATE OF NEW YORK

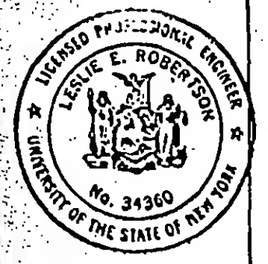
SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT	TITLE	DATE
THE WORLD TRADE CENTER	FLOOR PANELS	Printed 9/ 2/69

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7-AB5-	5	6/17/68
7-AB5-	6	6/17/68
7-AB5-	7	6/17/68
7-AB5-	8	6/17/68
7-AB5-	8.1	6/17/68
7-AB5-	8.2	6/17/68
7-AB5-	9	6/17/68
7-AB5-	10	6/17/68
7-AB5-	11	6/17/68
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7-AB5-	17	6/17/68
7-AB5-	18	6/17/68
7-AB5-	19	6/17/68
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2-C/2	6/13/69
7-C/1	5/13/69
7-C/2	6/13/69
8-C/1	5/14/69
8-C/2	6/13/69
9-C/1	5/20/69
9-C/2	6/13/69
10-C/1	5/14/69
10-C/2	6/13/69
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SKILLING-HELLE-CHRISTIANSEN-ROBERTSON — Civil & Structural Engineers

PROJECT	TITLE	DATE
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7-ABS-	4	10/11/68
7-ABS-	4.1	10/18/67
7-ABS-	5	6/17/68
7-ABS-	6	6/17/68
7-ABS-	7	6/17/68
7-ABS-	8	6/17/68
7-ABS-	8.1	6/17/68
7-ABS-	8.2	6/17/68
7-ABS-	9	6/17/68
7-ABS-	10	6/17/68
7-ABS-	11	6/17/68
7-ABS-	12	6/17/68
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LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
No. 34360
UNIVERSITY OF THE STATE OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON Structural & Civil Engineers

PROJECT THE WORLD TRADE CENTER | TITLE FLOOR PANELS | DATE 4/ 1/69

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7-AB5-	6	6/17/68
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7-AB5-	9	6/17/68
7-AB5-	10	6/17/68
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LICENSED PROFESSIONAL LESLIE E. ROBERTSON No. 3630 UNIVERSITY OF THE STATE OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT	TITLE	DATE
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7-AB5- 5 6/17/68
7-AB5- 6 6/17/68
7-AB5- 7 6/17/68
7-AB5- 8 6/17/68
7-AB5- 8.1 6/17/68
7-AB5- 8.2 6/17/68
7-AB5- 9 6/17/68
7-AB5- 10 6/17/68
7-AB5- 11 6/17/68
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7-AB5- 14 6/17/68
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SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT	TITLE	DATE
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7-AB5-	4.1	10/16/67
7-AB5-	5	6/17/68
7-AB5-	6	6/17/68
7-AB5-	7	6/17/68
7-AB5-	8	6/17/68
7-AB5-	8.1	6/17/68
7-AB5-	8.2	6/17/68
7-AB5-	9	6/17/68
7-AB5-	10	6/17/68
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LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
REGISTERED ARCHITECT OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS | DATE: 2/3/69

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7-AB5-	6	6/17/68
7-AB5-	7	6/17/68
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7-AB5-	9	6/17/68
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7-AB5-	11	6/17/68
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REGISTERED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
No. 5430

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS AND TRUSS DETAILS - DATE PAGE | DATE: 11/26/71

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7- B8- 3.5	3/19/69	7- B8-10.6	3/19/69			
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7- B8- 3.10	3/19/69	7- B8-12.1	3/19/69			
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LICENSED PROFESSIONAL ENGINEER — LESLIE E. ROBERTSON — No. 24360 — UNIVERSITY OF THE STATE OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANELS AND TRUSS DETAILS - DATE PAGE | DATE: 10/ 1/69

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7-AB1-55	Floor Trusses - Section A-A & B-B

GENERAL NOTES:

1. All material, including detail material not shown on these drawings to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated [ ] in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 7 Kips (7). See Drawing 7-AB2-1 for additional notes for additional shear forces at trusses C32 and 24T.

4. Shear transfer connections and devices shall be proportioned for the indicated horizontal shear force [ ] acting simultaneously with the horizontal shear force requirements indicated in the drawings for 24T trusses. Such connections and devices shall be proportioned for shear forces from C32 & 24T trusses of both positive sign and of negative sign.

5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

6. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

7. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 4.5 Kips.

8. All 32T trusses shall be cambered to a parabolic camber symmetrical about mid-span. Camber diagrams indicate required camber at mid-span.

9. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

Vertical shear forces between adjacent panels are indicated in truss elevations as "V = ". Where no force is given, shear transfer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Connections shall be proportioned for a shear force of both positive sign and negative sign.

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LICENSED PROFESSIONAL ENGINEER — LESLIE E. ROBERTSON — No. 54340 — UNIVERSITY OF THE STATE OF NEW YORK

GENERAL NOTES:

1. All material, including detail material not shown on these drawings to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated ( ) in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 7 Kips (7). See Drawing 7-AB2-1 for additional notes for additional shear forces at trusses C32 and 24T.

4. Shear transfer connections and devices shall be proportioned for the indicated horizontal shear force ( ) acting simultaneously with the horizontal shear force requirements indicated in the drawings for 24T trusses. Such connections and devices shall be proportioned for shear forces from C32 & 24T trusses of both positive sign and of negative sign.

5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

6. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

7. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 4.5 Kips.

8. All 32T trusses shall be cambered to a parabolic camber aymmetrical about mid-span. Camber diagrams indicate required camber at mid-span.

9. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

Vertical shear forces between adjacent panels are indicated in truss elevations as "V = ". Where no force is given, shear tranafer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Conneccions shall be proportioned for a shear force of both positive sign and negative sign.

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LICENSED PROFESSIONAL ENGINEER LESLIE E. ROBERTSON NO. 34306 UNIVERSITY OF THE STATE OF NEW YORK

GENERAL NOTES:

1. All material, including detail material not shown on these drawings to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated ( ) in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 7 Kips (7). See Drawing 7-AB2-1 for additional notes for additional shear forces at trusses C32 and 24T.

4. Shear transfer connections and devices shall be proportioned for the indicated horizontal shear force ( ) acting simultaneously with the horizontal shear force requirements indicated in the drawings for 24T trusses. Such connections and devices shall be proportioned for shear forces from C32 & 24T trusses of both positive sign and of negative sign.

5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

6. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

7. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 4.5 Kips.

8. All 32T trusses shall be cambered to a parabolic camber symmetrical about mid-span. Camber diagrams indicate required camber at mid-span.

9. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

Vertical shear forces between adjacent panels are indicated in truss elevations as "V = ". Where no force is given, shear transfer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Connections shall be proportioned for a shear force of both positive sign and negative sign.

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LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
No. 34360
UNIVERSITY OF THE STATE OF NEW YORK

GENERAL NOTES:

1. All material, including detail material not shown on these drawings to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated ( ) in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 7 Kips (7). See Drawing 7-AB2-1 for additional notes for additional shear forces at trusses C32 and 24T.

4. Shear transfer connections and devices shall be proportioned for the indicated horizontal shear force ( ) acting simultaneously with the horizontal shear force requirements indicated in the drawings for 24T trusses. Such connections and devices shall be proportioned for shear forces from C32 & 24T trusses of both positive sign and of negative sign.

5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

6. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

7. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 4.5 Kips.

8. All 32T trusses shall be cambered to a parabolic camber symmetrical about mid-span. Camber diagrams indicate required camber at mid-span.

9. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

   Vertical shear forces between adjacent panels are indicated in truss elevations as "V = ". Where no force is given, shear transfer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Connections shall be proportioned for a shear force of both positive sign and negative sign.

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GENERAL NOTES:

1. All material, including detail material not shown on these dwgs., to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated [ ] in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 7 Kips ([7]). See Dwg. 7-AB2-1 for additional notes for additional shear forces at trusses C32 and 24T.

4. Shear transfer connections and devices shall be proportioned for the indicated horizontal shear force ([ ]) acting simultaneously with the horizontal shear force requirements indicated in the drawings for 24T trusses. Such connections and devices shall be proportioned for shear forces from C32 & 24T trusses of both positive sign and of negative sign.

5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

6. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

7. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 4.5 Kips.

8. All 32T trusses shall be cambered to a parabolic camber symmetrical about mid-span. Camber diagrams indicate required camber at mid-span.

9. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

10. Vertical shear forces between adjacent panels are indicated in truss elevations as "V = ". Where no force is given, shear transfer connection shall be proportioned for a

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GENERAL NOTES:

1. All material, including detail material not shown on these dwgs., to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated [ ] in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 7 Kips ([7]). See Dwg. 7-AB2-1 for additional notes for additional shear forces at trusses C32 and 24T.

4. Shear transfer connections and devices shall be proportioned for the indicated horizontal shear force ([ ]) acting simultaneously with the horizontal shear force requirements indicated in the drawings for 24T trusses. Such connections and devices shall be proportioned for shear forces from C32 & 24T trusses of both positive sign and of negative sign.

5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

6. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

7. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 4.5 Kips.

8. All 32T trusses shall be cambered to a parabolic camber symmetrical about mid-span. Camber diagrams indicate required camber at mid-span.

9. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

10. Vertical shear forces between adjacent panels are indicated in truss elevations as "V = ". Where no force is given, shear transfer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Connections shall be proportioned for a shear force of both positive sign and

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GENERAL NOTES

1. All material, including detail material not shown on these dwgs., to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated [ ] in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 5 Kips ([5]). See Dwg. 7-AB1-1 for additional notes for additional shear forces.

4. See Dwg. 7-AB1-1 for notes regarding the joint occurrence of longitudinal and transverse horizontal shear force.

5. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

6. Vertical shear forces between adjacent panels are indicated in truss elevations are "V = ". Where no force is given, shear transfer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Connections shall be proportioned for a shear force of both positive sign and negative sign.

7. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

8. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

9. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 2.2 Kips.

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LESLIE E. ROBERTSON — LICENSED PROFESSIONAL ENGINEER — No. 34340 — STATE OF NEW YORK

GENERAL NOTES

1. All material, including detail material not shown on these dwgs., to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated [ ] in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 5 Kips ([5]). See Dwg. 7-AB1-1 for additional notes for additional shear forces.

4. See Dwg. 7-AB1-1 for notes regarding the joint occurrence of longitudinal and transverse horizontal shear force.

5. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

6. Vertical shear forces between adjacent panels are indicated in truss elevations are "V = ". Where no force is given, shear transfer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Connections shall be proportioned for a shear force of both positive sign and negative sign.

7. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

8. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

9. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 2.2 Kips.

SKILLING - HELLE - CHRISTIANSEN - ROBERTSON — Structural & Civil Engineers

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LESLIE E. ROBERTSON LICENSED PROFESSIONAL ENGINEER No. 34360 STATE OF NEW YORK

GENERAL NOTES

1. All material, including detail material not shown on these dwgs., to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated [ ] in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 5 Kips ([5]). See Dwg. 7-AB1-1 for additional notes for additional shear forces.

4. See Dwg. 7-AB1-1 for notes regarding the joint occurrence of longitudinal and transverse horizontal shear force.

5. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

6. Vertical shear forces between adjacent panels are indicated in truss elevations are "V = ". Where no force is given, shear transfer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Connections shall be proportioned for a shear force of both positive sign and negative sign.

7. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

8. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

9. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 2.2 Kips.

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LESLIE E. ROBERTSON — REGISTERED PROFESSIONAL ENGINEER — No. 34360 — STATE OF NEW YORK

GENERAL NOTES

1. All material, including detail material not shown on these dwgs., to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated [ ] in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 5 Kips ([5]). See Dwg. 7-AB1-1 for additional notes for additional shear forces.

4. See Dwg. 7-AB1-1 for notes regarding the joint occurrence of longitudinal and transverse horizontal shear force.

5. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

6. Vertical shear forces between adjacent panels are indicated in truss elevations are "V = ". Where no force is given, shear transfer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Connections shall be proportioned for a shear force of both positive sign and negative sign.

7. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

8. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

9. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 2.2 Kips.

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LICENSED PROFESSIONAL ENGINEER LESLIE E. ROBERTSON No. 34360 STATE OF NEW YORK

GENERAL NOTES

1. All material, including detail material not shown on these dwgs., to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical.

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated [ ] in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 5 Kips ([5]). See Dwg. 7-AB1-1 for additional notes for additional shear forces.

4. See Dwg. 7-AB1-1 for notes regarding the joint occurrence of longitudinal and transverse horizontal shear force.

5. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice conection shall be proportioned for a minimum force of 23 Kips (P = 23).

6. Vertical shear forces between adjacent panels are indicated in truss elevations are "V = ". Where no force is given, shear transfer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Connections shall be proportioned for a shear force of both positive sign and negative sign.

7. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

8. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

9. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of 2.2 Kips.

SKILLING - HELLE - CHRISTIANSEN - ROBERTSON — Structural & Civil Engineers

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GENERAL NOTES

1. All material, including detail material not shown on these dwgs., to be Fy = 50 unless otherwise noted.

2. All floor truss angles have short legs vertical,

3. Horizontal shear forces between truss panel points and the composite concrete slab are indicated [ ] in the truss elevations. Where no shear force is given, the connection shall be proportioned for a force of 5 Kips ([5]). See Dwg. 7-AB1-1 for additional notes for additional shear forces.

4. See Dwg. 7-AB1-1 for notes regarding the joint occurrence of longitudinal and transverse horizontal shear force.

5. Chord stresses are indicated in truss elevations as "P = ". Where no force is given each truss chord splice connection shall be proportioned for a minimum force of 23 Kips (P = 23).

6. Vertical shear forces between adjacent panels are indicated in truss elevations are "V = ". Where no force is given, shear transfer connection shall be proportioned for a minimum force of 5 Kips (V = 5). Connections shall be proportioned for a shear force of both positive sign and negative sign.

7. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

8. End reactions of each truss are indicated in truss elevations as "R = ". Where no force is given, end connections shall be proportioned for a minimum reaction of 9 Kips (R = 9).

9. Truss shear, indicating minimum requirements for each truss web member, is tabulated across the bottom of the truss elevations. Where no shear is given, web members and their connections shall be proportioned for a minimum shear of (2.2) Kips.

REGISTERED PROFESSIONAL ENGINEER — LESLIE E. ROBERTSON — No. 24360 — STATE OF NEW YORK

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GENERAL NOTES:

1. Material for steel deck supporting members L 3 x 2 to be Fy = 50 unless otherwise noted.

2. Deck beam angles and miscellaneous angles called for on floor panel drawings have long legs vertical.

3. Except as otherwise indicated, all L 3 x 2 deck supports shall be continuous.

4. Numbers shown on floor panel drawings thus (.) are vertical interaction forces in Kips between each C32 truss and the 24T truss.

   Where no interaction force is given, the connection shall be proportioned for a minimum connection of 5 Kips. All connections shall be proportioned for a tensile as well as a compressive interaction force of the specified value.

5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

6. Connection of L 3 x 2 deck support beams to C32 trusses shall be proportioned for a minimum reaction of 6 Kips.

LICENSED PROFESSIONAL ENGINEER — LESLIE E. ROBERTSON — NO. 34360 — UNIVERSITY OF THE STATE OF NEW YORK

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LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
No. 34360
UNIVERSITY OF THE STATE OF NEW YORK

GENERAL NOTES:

1. Material for steel deck supporting members ∠ 3 x 2 to be Fy = 50 unless otherwise noted.

2. Deck beam angles and miscellaneous angles called for on floor panel drawings have long legs vertical.

3. Except as otherwise indicated, all ∠ 3 x 2 deck supports shall be continuous.

4. Numbers shown on floor panel drawings thus ( ) are vertical interaction forces in Kips between each C32 truss and the 24T truss.

   Where no interaction force is given, the connection shall be proportioned for a minimum connection of 5 Kips. All connections shall be proportioned for a tensile as well as a compressive interaction force of the specified value.

5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

6. Connection of ∠ 3 x 2 deck support beams to C32 trusses shall

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7-AB3-79 Floor Panel C7
7-AB3-80 Floor Panel CR7

GENERAL NOTES:

1. Material for steel deck supporting members L 3 x 2 to be Fy = 50 unless otherwise noted.

2. Deck beam angles and miscellaneous angles called for on floor panel drawings have long legs vertical.

3. Except as otherwise indicated, all L 3 x 2 deck supports shall be continuous.

4. Numbers shown on floor panel drawings thus ( ) are vertical interaction forces in Kips between each C32 truss and the 24T truss.

   Where no interaction force is given, the connection shall be proportioned for a minimum connection of 5 Kips. All connections shall be proportioned for a tensile as well as a compressive interaction force of the specified value.

5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

6. Connection of L 3 x 2 deck support beams to C32 trusses shall be proportioned for a minimum reaction of 6 Kips.

LICENSED PROFESSIONAL ENGINEER LESLIE E. ROBERTSON No. 34360 UNIVERSITY OF THE STATE OF NEW YORK

SKILLING - HELLE - CHRISTIANSEN - ROBERTSON Structural & Civil Engineers

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7-AB3- 4 Floor Panel A1 G/E & A1 J/G
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7-AB3-55 Floor Panel BR8
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7-AB3-79 Floor Panel C7
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LICENSED PROFESSIONAL ENGINEER LESLIE E. ROBERTSON No. 34360 UNIVERSITY OF THE STATE OF NEW YORK

GENERAL NOTES:

1. Material for steel deck supporting members ∠ 3 x 2 to be Fy = 50 unless otherwise noted.

2. Deck beam angles and miscellaneous angles called for on floor panel drawings have long legs vertical.

3. Except as otherwise indicated, all ∠ 3 x 2 deck supports shall be continuous.

4. Numbers shown on floor panel drawings thus ( ) are vertical interaction forces in Kips between each C32 truss and the 24T truss.

   Where no interaction force is given, the connection shall be proportioned for a minimum connection of 5 Kips. All connections shall be proportioned for a tensile as well as a compressive interaction force of the specified value.

5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the Contractor. All connections, details and the like are subject to approval by the Engineer.

6. Connection of ∠ 3 x 2 deck support beams to C32 trusses shall

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7-AB3- 9 Floor Panel D1
7-AB3- 10 Floor Panel DR1
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7-AB3- 78 Floor Panel CR6
7-AB3- 79 Floor Panel C7
7-AB3- 80 Floor Panel CR7

REGISTERED PROFESSIONAL ENGINEER LESLIE E. ROBERTSON No. 24360 STATE OF NEW YORK

GENERAL NOTES:

1. Material for steel deck supporting members ∠3x2 to be Fy = 50 unless otherwise noted.
2. Deck beam angles and misc. angles called for on floor panel dwgs. have long legs vertical.
3. Except as otherwise indicated, all ∠3x2 deck supports shall be continuous.
4. Numbers shown on floor panel drawings thus ( ) are vertical interaction forces in Kips between each C32 truss and the 24T truss.
   Where no interaction force is given, the connection shall be proportioned for a minimum connection of 5 Kips. All connections shall be proportioned for a tensile as well as a compressive interaction force of the specified value.
5. Contractor shall supply representative calculations and/or

SKILLING - HELLE - CHRISTIANSEN - ROBERTSON Structural & Civil Engineers

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7-AB3- 76 Floor Panel FR5
7-AB3- 77 Floor Panel C6
7-AB3- 78 Floor Panel CR6
7-AB3- 79 Floor Panel C7
7-AB3- 80 Floor Panel CR7

GENERAL NOTES:

1. Material for steel deck supporting members ∠3x2 to be Fy = 50 unless otherwise noted.
2. Deck beam angles and misc. angles called for on floor panel dwgs. have long legs vertical.
3. Except as otherwise indicated, all ∠3x2 deck supports shall be continuous.
4. Numbers shown on floor panel drawings thus ( ) are vertical interaction forces in Kips between each C32 truss and the 24T truss.
   Where no interaction force is given, the connection shall be proportioned for a minimum connection of 5 Kips. All connections shall be proportioned for a tensile as well as a compressive interaction force of the specified value.
5. Contractor shall supply representative calculations and/or test results for each connection proportioned by the

LESLIE E. ROBERTSON ... No. 24300 ... STATE OF NEW YORK

SKILLING - HELLE - CHRISTIANSEN - ROBERTSON — Structural & Civil Engineers

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7-AB4-18.1	Floor Panel - Detail E30,
7-AB4-18.2	Floor Panel - Detail E30, E34, E34A
7-AB4-18.3	Floor Panel - Detail E34A
7-AB4-18.4	Floor Panel - Detail E34B
7-AB4-18.5	Floor Panel - Detail E31
7-AB4-19	Floor Panel - Detail E38 & E39
7-AB4-20	Not Used
7-AB4-21	Floor Panel - Detail E40, E40A, E45, E45A
7-AB4-22	Floor Panel - Detail E41, E41A, E41C - E41E
7-AB4-22.1	Floor Panel - Detail E41B & E46
7-AB4-23	Floor Panel - Detail E42
7-AB4-24	Floor Panel - Detail E43
7-AB4-25	Floor Panel - Detail E44
7-AB4-26	Not Used
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7-AB4-41.1	Floor Panel - Detail E68
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7-AB4-44	Floor Panel - Detail E61 - E67
7-AB4-45, 45.1	Floor Panel - Detail E65
7-AB4-46	Floor Panel - Detail E60 & E68
7-AB4-46.1	Floor Panel - Detail E68A
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7-AB4-61	Floor Panel - Detail E77A
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NOTES: 1. The Contractor shall detail all connections, erection aids and all else required for the fabrication, assembly and erection of the pre-fabricated floor panels. Steel deck and power/telephone cell details shall be done by the Contractor for WTC Contract 222.00 -- all other details by the Contractor for WTC Contract 221.00. Details shall incorporate the geometry requirements and specific detail requirements where indicated in Drawing Book 7. Details shall be proportioned to transmit the loads indicated in Drawing Book 7 and all other loads required for assembly and for erection.

2. The Contractor shall submit details to the

LICENSED PROFESSIONAL ENGINEER — LESLIE E. ROBERTSON — No. 34360 — STATE OF NEW YORK

SKILLING - HELLE - CHRISTIANSEN - ROBERTSON — Structural & Civil Engineers

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7-AB4-36	Floor Panel - Detail E56
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7-AB4-41.1	Floor Panel - Detail E68
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7-AB4-48	Floor Panel - Detail E61 - E64
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7-AB4-49.1	Floor Panel - Detail E68 & E68A
7-AB4-49.2	Floor Panel - Detail E69
7-AB4-5D thru 7-AB4-52	Floor Panel - Detail E70 thru E73
7-AB4-53	Floor Panel - Detail E74
7-AB4-54	Floor Panel - Detail E75
7-AB4-55	Floor Panel - Detail E75A
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7-AB4-57	Floor Panel - Detail E76
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7-AB4-59	Floor Panel - Detail E76A, E76B, E77
7-AB4-60	Floor Panel - Detail E76, E76A, E76B
7-AB4-61	Floor Panel - Detail E77A
7-AB4-62	Floor Panel - Detail E78, E79

NOTES:
1. The Contractor shall detail all connections, erection aids and all else required for the fabrication, assembly and erection of the pre-fabricated floor panels. Steel deck and power/telephone cell details shall be done by the Contractor for WTC Contract 222.0D -- all other details by the Contractor for WTC Contract 221.0D. Details shall incorporate the geometry requirements and specific detail requirements where indicated in Drawing Book 7. Details shall be proportioned to transmit the loads indicated in Drawing Book 7 and all other loads required for assembly and for erection.
2. The Contractor shall submit details to the Engineer for review prior to performing any work.

LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
No. 34360
UNIVERSITY OF THE STATE OF NEW YORK

SKILLING - HELLE - CHRISTIANSEN - ROBERTSON — Structural & Civil Engineers

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7-AB4-11.1	Floor Panel - Detail E12, E13
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7-AB4-11.3	Floor Panel - Detail E12, E13, E17, E-18
7-AB4-12	Floor Panel - Detail E16
7-AB4-13	Floor Panel - Detail E17
7-AB4-13.1	Floor Panel - Detail E18
7-AB4-13.2	Floor Panel - Detail E17 & E18
7-AB4-14	Floor Panel - Detail E19, E21, E22
7-AB4-14.1	Floor Panel - Detail E20
7-AB4-15	Floor Panel - Detail E23, E24
7-AB4-15.1 15.2	Floor Panel - Detail E25, E26, E26A
7-AB4-16	Floor Panel - Detail E27 & E27A
7-AB4-17 thru 7-AB4-17.7	Floor Panel - Detail E28 - E28E
7-AB4-18	Floor Panel - Detail E34, E35, E35A
7-AB4-18.1	Floor Panel - Detail E30,
7-AB4-18.2	Floor Panel - Detail E30, E34, E34A
7-AB4-18.3	Floor Panel - Detail E34A
7-AB4-18.4	Floor Panel - Detail E34B
7-AB4-18.5	Floor Panel - Detail E31
7-AB4-19	Floor Panel - Detail E38 & E39
7-AB4-20	Not Used
7-AB4-21	Floor Panel - Detail E40, E40A, E45, E45A
7-AB4-22	Floor Panel - Detail E41, E41A, E41C - E41E
7-AB4-22.1	Floor Panel - Detail E41B & E46
7-AB4-23	Floor Panel - Detail E42
7-AB4-24	Floor Panel - Detail E43
7-AB4-25	Floor Panel - Detail E44
7-AB4-26	Not Used
7-AB4-27	Not Used
7-AB4-28	Floor Panel - Detail E47 & E48
7-AB4-28.1	Floor Panel - Detail E47A
7-AB4-36	Floor Panel - Detail E56
7-AB4-37	Floor Panel - Detail E57
7-AB4-38	Floor Panel - Detail E58, E59, E60
7-AB4-38.1 & 38.2,38.3	Floor Panel - Detail E58, E59, E59A
7-AB4-39	Floor Panel - Detail E60
7-AB4-40	Floor Panel - Detail E61 - E67
7-AB4-41	Floor Panel - Detail E68A
7-AB4-41.1	Floor Panel - Detail E68
7-AB4-42	Floor Panel - Detail E61 - E67
7-AB4-42.1	Damper Connection - Detail E61 - E67
7-AB4-43	Floor Panel - Detail E60, E68, E68A
7-AB4-44	Floor Panel - Detail E61 - E67
7-AB4-45, 45.1	Floor Panel - Detail E65
7-AB4-46	Floor Panel - Detail E60 & E68
7-AB4-46.1	Floor Panel - Detail E68A
7-AB4-47	Floor Panel - Detail E60 - E67
7-AB4-47.1	Floor Panel - Detail E68 & E68A
7-AB4-48	Floor Panel - Detail E61 - E64
7-AB4-49	Floor Panel - Detail E60, E66, E67
7-AB4-49.1	Floor Panel - Detail E68 & E68A
7-AB4-49.2	Floor Panel - Detail E69
7-AB4-50 thru 7-AB4-52	Floor Panel - Detail E70 thru E73
7-AB4-53	Floor Panel - Detail E74
7-AB4-54	Floor Panel - Detail E75
7-AB4-55	Floor Panel - Detail E75A
7-AB4-56	Floor Panel - Detail E76
7-AB4-57	Floor Panel - Detail E76
7-AB4-58	Floor Panel - Detail E76A, E76B
7-AB4-59	Floor Panel - Detail E76A, E76B, E77
7-AB4-60	Floor Panel - Detail E76, E76A, E76B
7-AB4-61	Floor Panel - Detail E77A
7-AB4-62	Floor Panel - Detail E78, E79

NOTES:

1. The Contractor shall detail all connections, erection aids and all else required for the fabrication, assembly and erection of the pre-fabricated floor panels. Steel deck and power/telephone cell details shall be done by the Contractor for WTC Contract 222.00 -- all other details by the Contractor for WTC Contract 221.00. Details shall incorporate the geometry requirements and specific detail requirements where indicated in Drawing Book 7. Details shall be proportioned to transmit the loads indicated in Drawing Book 7 and all other loads required for assembly and for erection.
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LICENSED PROFESSIONAL ENGINEER — LESLIE E. ROBERTSON — No. 34360 — STATE OF NEW YORK

SKILLING - HELLE - CHRISTIANSEN - ROBERTSON Structural & Civil Engineers

PROJECT	TITLE	DATE
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7-AB4- 1	Floor Panel Details, Index & General Notes
7-AB4- 2	Schedule of Details - Tower A
7-AB4- 2.1	Schedule of Details - Tower A
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7-AB4- 3	Floor Panel - Detail E1-E3
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7-AB4- 4	Floor Panel - Detail E5 & E5A
7-AB4- 5	Floor Panel - Detail E6
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7-AB4-10	Floor Panel - Detail E11
7-AB4-11	Floor Panel - Detail E12
7-AB4-11.1	Floor Panel - Detail E12, E13
7-AB4-11.2	Floor Panel - Detail E13
7-AB4-11.3	Floor Panel - Detail E12, E13, E17, E-18
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7-AB4-14	Floor Panel - Detail E19, E21, E22
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7-AB4-15.1 15.2	Floor Panel - Detail E25, E26, E26A
7-AB4-16	Floor Panel - Detail E27 & E27A
7-AB4-17 thru 7-AB4-17.7	Floor Panel - Detail E28 - E28E
7-AB4-18	Floor Panel - Detail E34, E35, E35A
7-AB4-18.1	Floor Panel - Detail E30,
7-AB4-18.2	Floor Panel - Detail E30, E34, E34A
7-AB4-18.3	Floor Panel - Detail E34A
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7-AB4-18.5	Floor Panel - Detail E31
7-AB4-19	Floor Panel - Detail E38 & E39
7-AB4-20	Not Used
7-AB4-21	Floor Panel - Detail E40, E40A, E45, E45A
7-AB4-22	Floor Panel - Detail E41, E41A, E41C - E41E
7-AB4-22.1	Floor Panel - Detail E41B & E46
7-AB4-23	Floor Panel - Detail E42
7-AB4-24	Floor Panel - Detail E43
7-AB4-25	Floor Panel - Detail E44
7-AB4-26	Not Used
7-AB4-27	Not Used
7-AB4-28	Floor Panel - Detail E47 & E48
7-AB4-36	Floor Panel - Detail E56
7-AB4-37	Floor Panel - Detail E57
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7-AB4-41	Floor Panel - Detail E68A
7-AB4-41.1	Floor Panel - Detail E68
7-AB4-42	Floor Panel - Detail E61 - E67
7-AB4-42.1	Damper Connection - Detail E61 - E67
7-AB4-43	Floor Panel - Detail E6D, E68, E68A
7-AB4-44	Floor Panel - Detail E61 - E67
7-AB4-45,45.1	Floor Panel - Detail E65
7-AB4-46	Floor Panel - Detail E60 & E68
7-AB4-46.1	Floor Panel - Detail E68A
7-AB4-47	Floor Panel - Detail E60 - E67
7-AB4-47.1	Floor Panel - Detail E68 & E68A
7-AB4-48	Floor Panel - Detail E61 - E64
7-AB4-49	Floor Panel - Detail E60, E66, E67
7-AB4-49.1	Floor Panel - Detail E68 & E68A
7-AB4-49.2	Floor Panel - Detail E69
7-AB4-50 thru 7-AB4-52	Floor Panel - Detail E70 thru E73
7-AB4-53	Floor Panel - Detail E74
7-AB4-54	Floor Panel - Detail E75
7-AB4-55	Floor Panel - Detail E75A
7-AB4-56	Floor Panel - Detail E76
7-AB4-57	Floor Panel - Detail E76
7-AB4-58	Floor Panel - Detail E76A, E76B
7-AB4-59	Floor Panel - Detail E76A, E76B, E77
7-AB4-60	Floor Panel - Detail E76, E76A, E76B
7-AB4-61	Floor Panel - Detail E77A
7-AB4-62	Floor Panel - Detail E78, E79

LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
No. 34360
UNIVERSITY OF THE STATE OF NEW YORK

NOTES:

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SKILLING - HELLE - CHRISTIANSEN - ROBERTSON Structural & Civil Engineers

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7-AB4- 3	Floor Panel - Detail E1-E3
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7-AB4- 5	Floor Panel - Detail E6
7-AB4- 6	Floor Panel - Detail E6
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7-AB4-10	Floor Panel - Detail E11
7-AB4-11	Floor Panel - Detail E12
7-AB4-11.1	Floor Panel - Detail E12, E13
7-AB4-11.2	Floor Panel - Detail E13
7-AB4-11.3	Floor Panel - Detail E12, E13, E17, E-18
7-AB4-12	Floor Panel - Detail E16
7-AB4-13	Floor Panel - Detail E17
7-AB4-13.1	Floor Panel - Detail E18
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7-AB4-15	Floor Panel - Detail E23, E24
7-AB4-15.1 15.2	Floor Panel - Detail E25, E26, E26A
7-AB4-16	Floor Panel - Detail E27 & E27A
7-AB4-17 thru 7-AB4-17.7	Floor Panel - Detail E28 - E28E
7-AB4-18	Floor Panel - Detail E34
7-AB4-18.1	Floor Panel - Detail E30
7-AB4-18.2	Floor Panel - Detail E30, E34
7-AB4-18.3	Floor Panel - Detail E34A
7-AB4-18.4	Floor Panel - Detail E34B
7-AB4-19	Floor Panel - Detail E38 & E39
7-AB4-20	Not Used
7-AB4-21	Floor Panel - Detail E40 & E45
7-AB4-22	Floor Panel - Detail E41 & E41A
7-AB4-22.1	Floor Panel - Detail E41B & E46
7-AB4-23	Floor Panel - Detail E42
7-AB4-24	Floor Panel - Detail E43
7-AB4-25	Floor Panel - Detail E44
7-AB4-26	Not Used
7-AB4-27	Not Used
7-AB4-28	Floor Panel - Detail E47 & E48
7-AB4-28.1	Floor Panel - Detail E47A
7-AB4-36	Floor Panel - Detail E56
7-AB4-37	Floor Panel - Detail E57
7-AB4-38	Floor Panel - Detail E58, E59, E60
7-AB4-38.1 & 38.2	Floor Panel - Detail E58, E59
7-AB4-39	Floor Panel - Detail E60
7-AB4-40	Floor Panel - Detail E61 - E67
7-AB4-41	Floor Panel - Detail E68A
7-AB4-41.1	Floor Panel - Detail E68
7-AB4-42	Floor Panel - Detail E61 - E67
7-AB4-42.1	Damper Connection - Detail E61 - E67
7-AB4-43	Floor Panel - Detail E60, E68, E68A
7-AB4-44	Floor Panel - Detail E61 - E67
7-AB4-45, 45.1	Floor Panel - Detail E65
7-AB4-46	Floor Panel - Detail E60 & E68
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7-AB4-48	Floor Panel - Detail E61 - E64
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7-AB4-55	Floor Panel - Detail E75A
7-AB4-56	Floor Panel - Detail E76
7-AB4-57	Floor Panel - Detail E76
7-AB4-58	Floor Panel - Detail E76A, E76B
7-AB4-59	Floor Panel - Detail E76A, E76B, E77
7-AB4-60	Floor Panel - Detail E76, E76A, E76B
7-AB4-61	Floor Panel - Detail E77A

LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
No. 34360
STATE OF NEW YORK

NOTES:
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SKILLING - HELLE - CHRISTIANSEN - ROBERTSON — Structural & Civil Engineers

PROJECT: THE WORLD TRADE CENTER | TITLE: FLOOR PANEL DETAILS - INDEX & GENERAL NOTES | DATE: 1-3-68

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7-AB4- 2.4	Schedule of Details - Tower B	7-AB4- 47	Floor Panel - Detail E60-E68
7-AB4- 3	Floor Panel - Detail E1-E3	7-AB4- 48	Floor Panel - Detail E61-E64
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7-AB4- 7	Floor Panel - Detail E8		
7-AB4- 8	Floor Panel - Detail E9 &		
7-AB4- 9	Floor Panel - Detail E10		
7-AB4- 10	Floor Panel - Detail E11		
7-AB4- 11	Floor Panel - Detail E12,E13,E14,E15		
7-AB4- 11.1	Floor Panel - Detail E12,E13,E14,E15		
7-AB4- 12	Floor Panel - Detail E16		
7-AB4- 13	Floor Panel - Detail E17 & E18		
7-AB4- 14	Floor Panel - Detail E19,E21,E22		
7-AB4- 14.1	Floor Panel - Detail E20		
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7-AB4- 18	Floor Panel - Detail E30,E31,E32,E33,E34, E35,E36,E37		
7-AB4- 19	Floor Panel - Detail E38 & E39		
7-AB4- 20	Not Used		
7-AB4- 21	Floor Panel - Detail E40 & E45		
7-AB4- 22	Floor Panel - Detail E41 & E46		
7-AB4- 23	Floor Panel - Detail E42		
7-AB4- 24	Floor Panel - Detail E43		
7-AB4- 25	Floor Panel - Detail E44		
7-AB4- 26	Not Used		
7-AB4- 27	Not Used		
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7-AB4- 31	Floor Panel - Detail E51A, E51B, E51C		
7-AB4- 32	Floor Panel - Detail E52A, E52B, E52C		
7-AB4- 33	Floor Panel - Detail E53		
7-AB4- 34	Floor Panel - Detail E54		
7-AB4- 35	Floor Panel - Detail E55		
7-AB4- 36	Floor Panel - Detail E56		
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7-AB4- 38	Floor Panel - Detail E58,E59,E60		
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LESLIE E. ROBERTSON — No. 34360 — REGISTERED PROFESSIONAL ENGINEER — STATE OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT: THE WORLD TRADE CENTER | TITLE: STEEL DECK - INDEX & GEN. NOTES | DATE: 4/ 1/69

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Prin. ed 5-1-69

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7-AB5- 4 POWER/TELEPHONE DETAILS
7-AB5- 4.1 POWER/TELEPHONE DETAILS
7-AB5- 5 SECTIONS A-DCOD
7-AB5- 6 SECTIONS E-H
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7-AB5- 8.1 SECTIONS U-X
7-AB5- 8.2 SECTIONS Y, YYGZ
7-AB5- 9 DETAIL K
7-AB5-10 DETAIL M
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GENERAL NOTES.

1. STEEL DECK TO BE 22 GAGE GALV.

2. ALL PLUG WELDS SHALL BE FURNISHED UNDER CONTRACT WTC 223.00 OR CONTRACT WTC 230.00, WHICH EVER IS APPLICABLE.

LICENSED PROFESSIONAL ENGINEER — LESLIE E. ROBERTSON — No. 34360 — STATE OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT	TITLE	DATE
THE WORLD TRADE CENTER	STEEL DECK - INDEX & GEN. NOTES	4/ 1/69

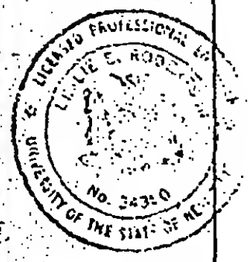
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7-AB5- 1 INDEX & GEN. NOTES
7-AB5- 2 TYPICAL DETAILS A-D
7-AB5- 3 TYPICAL DETAILS E-G
7-AB5- 3.1 TYPICAL DETAIL H
7-AB5- 3.2 TYPICAL DETAIL K
7-AB5- 4 POWER/TELEPHONE DETAILS
7-AB5- 4.1 POWER/TELEPHONE DETAILS
7-AB5- 5 SECTIONS A-D&DD
7-AB5- 6 SECTIONS E-H
7-AB5- 7 SECTIONS J-N
7-AB5- 8 SECTIONS P-T
7-AB5- 8.1 SECTIONS U-X
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7-AB5- 9 DETAIL K
7-AB5-10 DETAIL M
7-AB5-11 DETAIL N
7-AB5-12 DETAIL P
7-AB5-13 DETAIL Q
7-AB5-14 DETAIL R
7-AB5-15 DETAIL S
7-AB5-16 SUPPORTS
7-AB5-17 DETAIL T
7-AB5-18 DETAIL U
7-AB5-19 DETAIL VEH

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SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT	TITLE	DATE
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7-AB5- 4.1 POWER/TELEPHONE DETAILS
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7-AB5- 6 SECTIONS E-H
7-AB5- 7 SECTIONS J-N
7-AB5- 8 SECTIONS P-T
7-AB5- 8.1 SECTIONS U-X
7-AB5- 8.2 SECTIONS Y,YYGZ
7-AB5- 9 DETAIL K
7-AB5-10 DETAIL M
7-AB5-11 DETAIL N
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7-AB5-13 DETAIL Q
7-AB5-14 DETAIL R
7-AB5-15 DETAIL S
7-AB5-16 SUPPORTS
7-AB5-17 DETAIL T
7-AB5-18 DETAIL U
7-AB5-19 DETAIL VGW

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LICENSED PROFESSIONAL ENGINEER
LESLIE E. ROBERTSON
NO. 31260
UNIVERSITY OF THE STATE OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

PROJECT	TITLE	DATE
THE WORLD TRADE CENTER	STEEL DECK - INDEX & GEN. NOTES	2/ 3/69

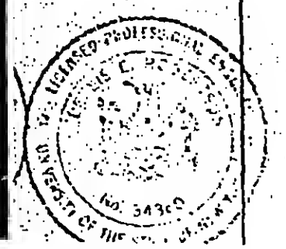
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GENERAL NOTES.

1. STEEL DECK TO BE 22 GAGE GALV.

2. ALL PLUG WELDS SHALL BE FURNISHED UNDER CONTRACT WTC 223.00 OR CONTRACT WTC 230.00, WHICH EVER IS APPLICABLE.



SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

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GENERAL NOTES.

1. STEEL DECK TO BE 22 GAGE GALV.
2. ALL PLUG WELDS SHALL BE FURNISHED UNDER CONTRACT WTC 223.00 OR CONTRACT WTC 230.00, WHICH EVER IS APPLICABLE.

LICENSED PROFESSIONAL ENGINEER — LESLIE E. ROBERTSON — NO. 34360 — STATE OF NEW YORK

SKILLING - HELLE - CHRISTIANSEN - ROBERTSON — Structural & Civil Engineers

PROJECT THE WORLD TRADE CENTER | TITLE STEEL DECK · INDEX & GENERAL NOTES | DATE 10-18-67

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GENERAL NOTES

1. Steel deck to be 22 gage galv.
2. All plug welds shall be furnished under Contract WTC 223.00 or Contract WTC 230.00, whichever is applicable.

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SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

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REGISTERED PROFESSIONAL ENGINEER No. 5436 STATE OF HAWAII

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

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REGISTERED PROFESSIONAL LAND SURVEYOR
LESLIE E. ROBERTSON
NO. 34360
UNIVERSITY OF THE STATE OF NEW YORK

SKILLING, HELLE, CHRISTIANSEN, ROBERTSON — Structural & Civil Engineers

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3-19-69
4-1-69
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GENERAL NOTES

1. All material, including detail material to be Fy = 50 except as noted. 1½" x 5/8" Straps (Detail 12), 15" & 18" channels (Detail 13) and deck support angles (Detail 12 & 16) to be Fy = 36.
2. All welding shall be accomplished using E70xx low hydrogen electrodes or other equivalent, approved welding process. Field welding shall be performed under Contract WTC-230.00.
3. All bolted connections shall be friction type connections. All bolts shall be ASTM A325 bolts except where ASTM A490 bolts are specifically called for in the drawings. Washer shall be used under both bolt head and nut at all locations with oversize or elongated holes, in accordance with the requirement of FB1, FB2 & FB3. Bolts and washers will be supplied under Contract WTC-234.00 and installed under Contract WTC-230.00.
4. All floor truss angles have long legs back to back or vertical unless noted (SV) in the drawings, in which case, short legs shall be back to back or vertical.
5. Forces in truss members are indicated thus in the drawings:
   Compression (-)
   Tension (+)
6. All 29T trusses shall be cambered to a parabolic camber. Camber diagrams 7-B7-26 indicates required camber at mid span.
7. Truss details are shown in 7-B8. See index 7-B8-1.
8. See SB-18, SB-19, SB-20 & SB-21 for location of trusses. See SB-18, SB-19, SB-20, SB-21 and this chapter of Drawing Book 7 (7-B7) for location of Details.
9. See SB-184, SB-185, SB-186 & SB-187 for Steel Deck Plans and Shear srud location and number. Steel deck shall be furnished under Contract WTC-222.00 and installed under Contract WTC-230.00. Shear studs shall be furnished and installed under Contract WTC-230.00
10. Seats for trusses at core perimeter are shown in Drawing Book 6 and furnished under Contract WTC-226.00. Seats for trusses at the exterior wall are shown in Drawing Book 4 and furnished under Contract WTC-214.00.

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1. All material, including detail material, to be Fy = 50 unless otherwise noted.
2. All floor truss angles long legs back to back or vertical unless noted (SV) in which case short legs shall be back to back or vertical.
3. All 29T trusses shall be cambered to a parabolic camber symmetrical about mid-span. Camber diagrams 7-B7-25 indicate required camber at mid-span.

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7-B8-8.4	DETAIL 8.4	7-B8-15.1	DETAIL 15.1
7-B8-8.5	DETAIL 8.5	7-B8-15.2	DETAIL 15.2
7-B8-8.6	DETAIL 8.6	7-B8-16.1	DETAIL 16.1-16.6
7-B8-8.7	DETAIL 8.7		
7-B8-8.8	DETAIL 8.8		
7-B8-8.9	DETAIL 8.9		
7-B8-8.10	DET. 8 SECT A, B		

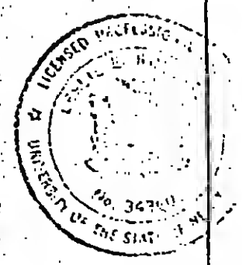
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